

# FORM A

## ESM 2204 Fall 2005 MECHANICS OF DEFORMABLE BODIES Final Exam

NAME

(print legibly)

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last

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first

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initial

**PLEDGE (signature): On my honor I have neither given nor received unauthorized aid on this test.**

### INSTRUCTIONS:

**Closed book, closed notes, 8.5" x 11" formula (only) sheet.**

- There are 11 questions on this exam - check for completeness.
- Please be sure to mark you Form letter on the op-scan form.
- Part I (54%) consists of 9 multiple-choice problems (each problem is worth equal credit).
- Part II (46%) consists of two work-out problems. Please complete all work in the space provided.

**Turn in your results in the following order:**

Exam questions (signed)

Formula sheet

### CHECK FOR COMPLETENESS

#### Answers:

##### Part I:

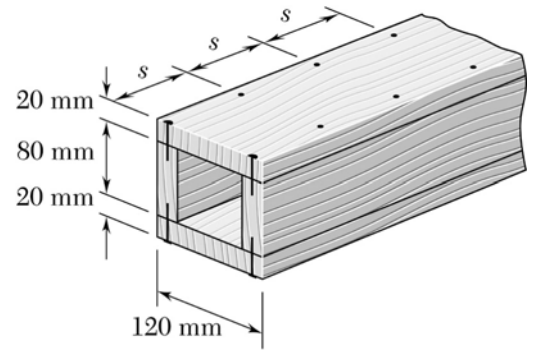
- 1) 1387 N
- 2) 3961 lb
- 3)  $(6wL + 5P)/16$
- 4) 7330 psi
- 5) 5.13 ksi
- 6) 0.0754 mm
- 7) 186 N
- 8) 41.7 kN
- 9) 12.38 ksi

##### Part II:

- 1)  $\delta_{@D} = 4 a \Delta T L / 3$
- 2)  $h = 14.55$  in

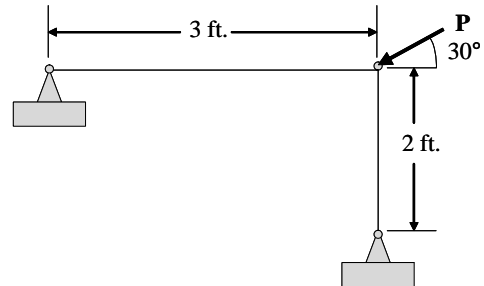
**PART I: MULTIPLE CHOICE PROBLEMS (6 Points each)**

1. A square box beam is made of two 20X80-mm planks and 20X120-mm planks nailed together as shown. Knowing that the spacing between the nails is  $s = 50$  mm and that the allowable shearing force in each nail is 300N, the largest allowable vertical shear in the beam is most nearly



- (a) 147 N
- (b) 694 N
- (c) 1387 N
- (d) 2770 N
- (e) 5540 N

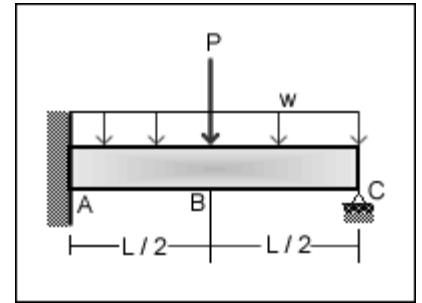
2. In the structure shown, each member has a diameter of 0.75 in and is made from steel ( $E = 29 \times 10^6$  psi). If a factor of safety of 2.0 against buckling is required, the maximum allowable load  $P$  is most nearly



- (a) 7720 lb
- (b) 3430 lb
- (c) 3960 lb
- (d) 15400 lb
- (e) 6860 lb

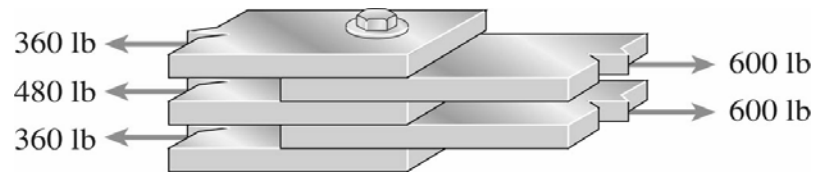
3. For the beam and loading condition shown, the reaction at support C is most nearly

- (a)  $(6wL + 5P)/16$
- (b)  $(5wL + 6P)/16$
- (c)  $(6wL + 5P)/8$
- (d)  $(5wL + 6P)/8$
- (e)  $(6wL + 5P)/4$



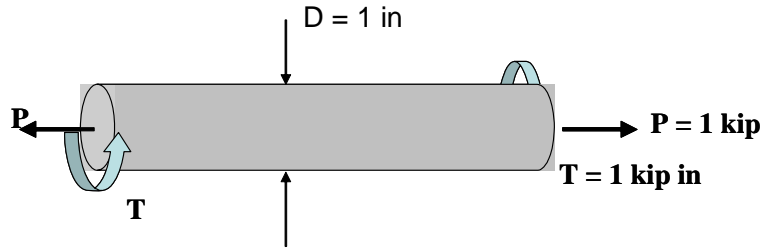
4. The connection shown below consists of five steel plates, each  $3/16$  in. thick, joined by a single  $1/4$  in. diameter bolt. The total load transferred between the plates is 1200 lb, distributed among the plates as shown. The largest shearing stress in the bolt is most nearly

- (a) 24400 psi
- (b) 9780 psi
- (c) 12200 psi
- (d) 7330 psi
- (e) 3670 psi



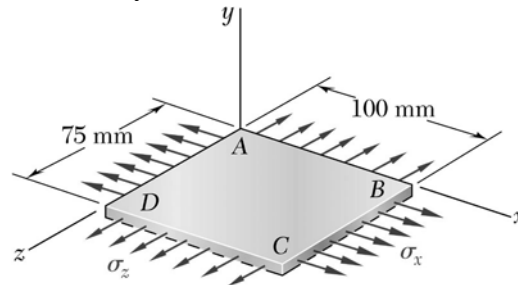
5. A cylindrical bar is subjected to the applied torque and axial load shown. The maximum shear stress in the bar is most nearly

- (a) 5.13 ksi
- (b) 5.09 ksi
- (c) 1.27 ksi
- (d) 0.64 ksi
- (e) 5.77 ksi



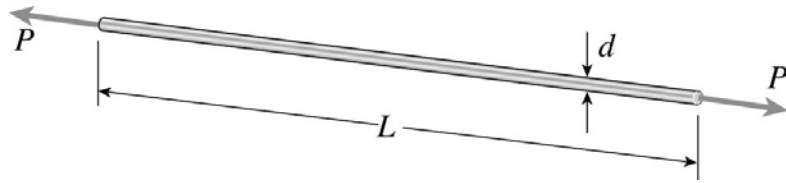
6. A fabric used in air-inflated structures is subjected to a biaxial loading that results in normal stresses  $\sigma_x = 120$  MPa and  $\sigma_z = 160$  MPa. Knowing that the properties of the fabric can be approximated as  $E = 87$  GPa and  $\nu = 0.34$ , the change in length of side  $AB$  is most nearly

- (a) 0.1503 mm
- (b) 0.200 mm
- (c) 0.1034 mm
- (d) 0.1379 mm
- (e) 0.0754 mm



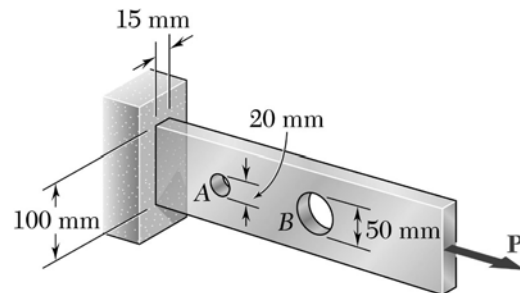
7. An aluminum wire having a diameter  $d = 2 \text{ mm}$  and length  $L = 3.8 \text{ m}$  is subjected to a tensile load  $P$  (see figure). The aluminum has a modulus of elasticity  $E = 75 \text{ GPa}$ . If the maximum permissible elongation of the wire is  $3.0 \text{ mm}$  and the allowable stress in tension is  $60 \text{ MPa}$ , the allowable load  $P_{max}$  is most nearly

- (a) 189 N
- (b) 186 N
- (c) 183 N
- (d) 180 N
- (e) 177 N



8. Knowing that  $\sigma_{all} = 120 \text{ MPa}$ , the maximum allowable value of the centric axial load  $\mathbf{P}$  is most nearly

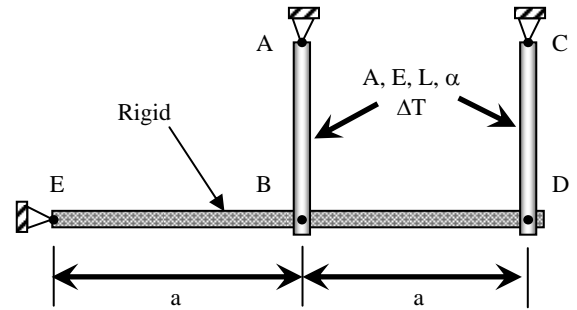
- (a) 54.3 kN
- (b) 41.7 kN
- (c) 143.9 kN
- (d) 90.0 kN
- (e) 35.3 kN



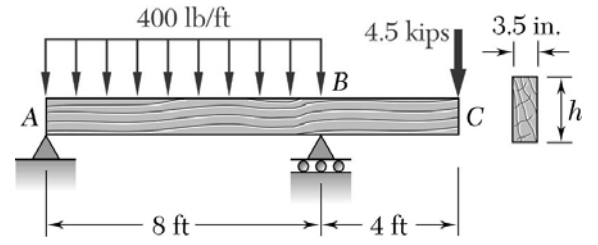
9. A spherical gas container having an outer diameter of 15 ft and a wall thickness of 0.90 in. is made of a steel for which  $E = 29 \times 10^6$  psi and  $\nu = 0.29$ . Knowing that the gage pressure in the container is increased from zero to 250 psi, the maximum normal stress in the container is most nearly
- (a) 55.3 ksi
  - (b) 49.5 ksi
  - (c) 24.8 ksi
  - (d) 12.38 ksi
  - (e) 6.19 ksi

**PART II: WORK OUT PROBLEMS (23 Points Each)**

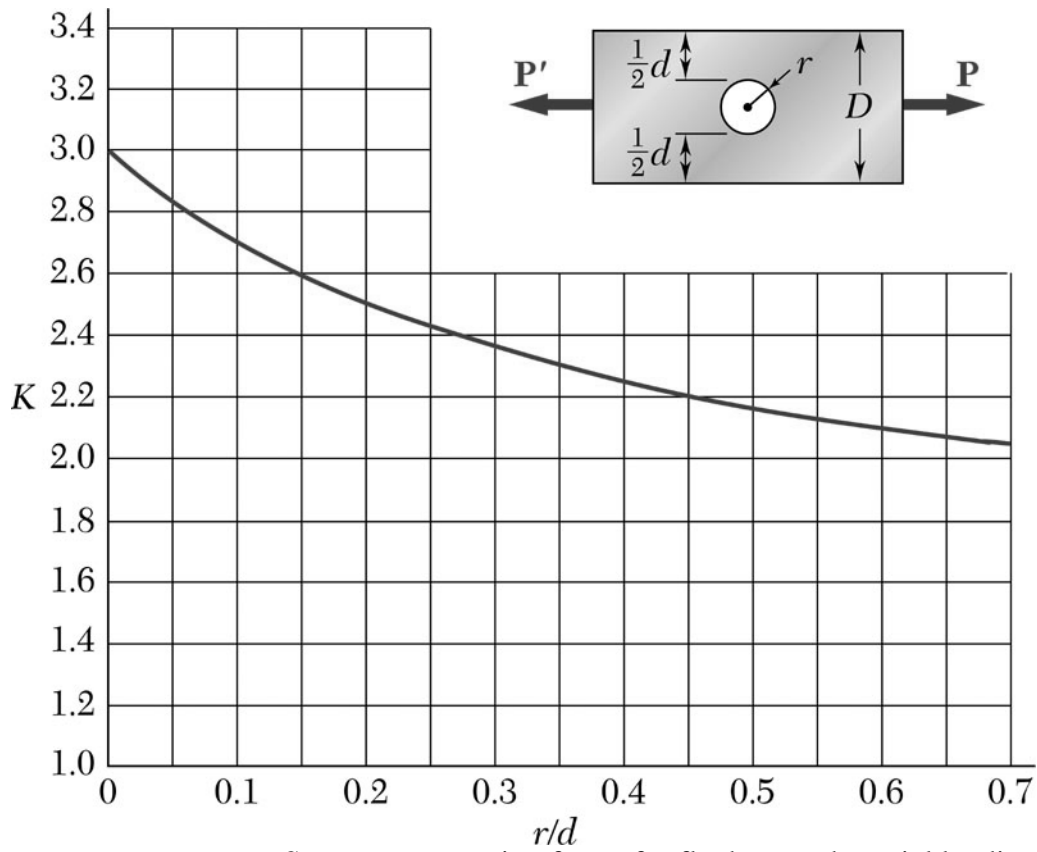
1. The rigid beam ED of length  $2a$  is supported by the two identical rods AB and CD as shown below. The rods are made from a material with a modulus of elasticity  $E$ , a coefficient of thermal expansion  $\alpha$ , cross-sectional area  $A$ , and length  $L$ . If the rods AB and CD are subjected to a temperature difference  $\Delta T$  ( $\Delta T > 0$ ), find the deflection of point D in terms of  $E$ ,  $\alpha$ ,  $A$ ,  $L$ ,  $a$ , and  $\Delta T$ .



2. A 12-ft-long overhanging timber beam  $AC$  with an 8-ft span  $AB$  is to be designed to support the distributed and concentrated loads shown. Knowing that timber of 4-in. nominal width (3.5-in. actual width) with a 1.75-ksi allowable stress is to be used, determine the minimum required depth  $h$  of the beam.

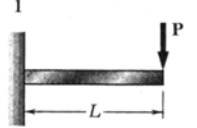
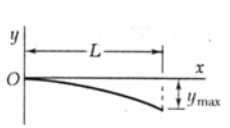
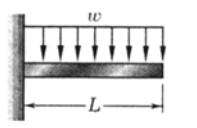
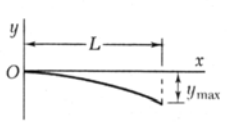
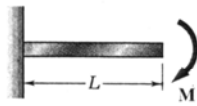
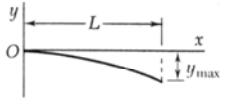
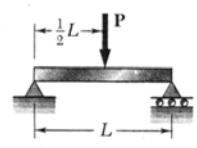
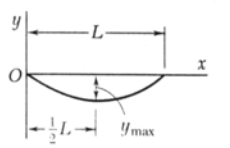
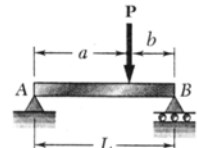
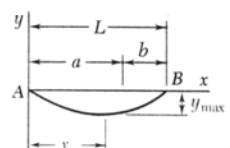
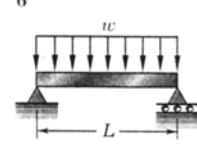
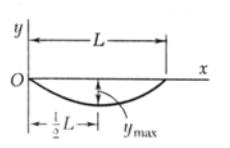
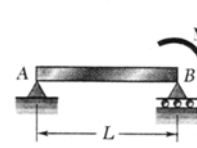
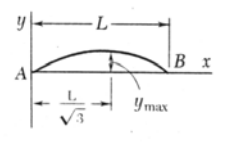






Stress concentration factor for flat bars under axial loading.

Note that the average stress must be computed across the narrowest section:  $\sigma_{ave} = P/t$  where  $t$  is the thickness of the bar.

Beam and Loading	Elastic Curve	Maximum Deflection	Slope at End	Equation of Elastic Curve
		$-\frac{PL^3}{3EI}$	$-\frac{PL^2}{2EI}$	$y = \frac{P}{6EI}(x^3 - 3Lx^2)$
		$-\frac{wL^4}{8EI}$	$-\frac{wL^3}{6EI}$	$y = -\frac{w}{24EI}(x^4 - 4Lx^3 + 6L^2x^2)$
		$-\frac{ML^2}{2EI}$	$-\frac{ML}{EI}$	$y = -\frac{M}{2EI}x^2$
		$-\frac{PL^3}{48EI}$	$\pm \frac{PL^2}{16EI}$	For $x \leq \frac{1}{2}L$ : $y = \frac{P}{48EI}(4x^3 - 3L^2x)$
		For $a > b$ : $-\frac{Pb(L^2 - b^2)^{3/2}}{9\sqrt{3}EIL}$ at $x_m = \sqrt{\frac{L^2 - b^2}{3}}$	$\theta_A = -\frac{Pb(L^2 - b^2)}{6EIL}$ $\theta_B = +\frac{Pa(L^2 - a^2)}{6EIL}$	For $x < a$ : $y = \frac{Pb}{6EIL}[x^3 - (L^2 - b^2)x]$ For $x = a$ : $y = -\frac{Pa^2b^2}{3EIL}$
		$-\frac{5wL^4}{384EI}$	$\pm \frac{wL^3}{24EI}$	$y = -\frac{w}{24EI}(x^4 - 2Lx^3 + L^3x)$
		$\frac{ML^2}{9\sqrt{3}EI}$	$\theta_A = +\frac{ML}{6EI}$ $\theta_B = -\frac{ML}{3EI}$	$y = -\frac{M}{6EIL}(x^3 - L^2x)$